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PROJECT: Yorktown Farms Subdivision

Town of Yorktown, NY

SCOPE: Drainage and Hydrology Report

DATE: December 6, 2004

INTRODUCTION:

The proposed construction of new roads and homes on this 43-acre site requires the study of the impacts on watercourses in and around the site. This study reviews the existing drainage conditions as well as the proposed improvements to provide measures that will be used to control any potential impacts due to stormwater runoff. For this report, a 34-lot plan has been studied. Approximately 13.8-acres of the site are located in the NYCDEP watershed.

METHODOLOGY:

The watersheds are divided into subareas, by topography, land use, and SCS soil hydrologic grouping. Tabulations of areas and descriptions are shown on the enclosed maps and tables. A summary of the watershed areas, composite curve numbers, and travel times is shown in Table 1.

The flows from the watersheds in the existing condition are computed to determine undeveloped peak runoff and runoff hydrographs at selected design points. The existing peak flows are presented in Table 2.

In the after development condition, the flows from the proposed development are computed by using the runoff curve numbers taken from TR-55. The watersheds are adjusted for the proposed grading of the site. The runoff flows are hydraulically routed for updated travel times, runoff diversions, and new storage structures as necessary. The resulting, proposed peak flows at each design point are presented in Table 2.

Maps have been prepared showing the existing and proposed watersheds and are included in this report.

The methods used are those presented in the US Army Corps. of Engineers HEC 1 computer program using a shortened printout for convenience. The 100, 50, 25, 10, 5, and 2-year frequency storms are studied. The 1 year storm is also studied to analyze Channel Protection (Cp_v), as per NYSDEC. The 1.3 inch rainfall is also studied to determine the Water Quality Volume (WQ_{v_l} . The SCS type III - 24-hour storm distribution (Westchester) is used throughout. Soil types and hydrologic groups are based on soil maps from the Westchester Soil and Water Board. Topographical mapping for the watersheds are taken from digital topography maps from the Town of Yorktown.

YORKTOWN FARMS SUBDIVISION

TOWN OF YORKTOWN WESTCHESTER COUNTY, NEW YORK

DRAINAGE AND HYDROLOGY REPORT

December 6, 2004

Prepared for

37 Croton Dam Road Corporation 37 Croton Dam Road Ossining, New York 10562

Consulting Engineers 13 Dove Court, Croton-on-Hudson, New York 10520

0.16

NA

0.11

0.15

0.17

0.04

0.28

DRAINAGE AREA NAME	EXISTING DRAINAGE AREA (ACRES)	PROPOSED DRAINAGE AREA (ACRES)	EXISTING CURVE NO.	PROPOSED CURVE NO.	EXISTING LAG (HOURS)	PROPOSED LAG (HOURS)
WS1	13.40	NA	65.29	NA	0.27	NA
WS1A	NA	1.53	NA	73.42	NA	0.12
WS1B	NA	4.48	NA	77.75	NA	0.13
WS1C	NA	3.05	NA	77.13	NA	0.11
WS1D	NA	3.65	NA	67.14	NA	0.11
WS2	1.61	1.65	66.00	70.00	0.16	0.16
WS3	4.25	4.19	75.85	74.56	0.20	0.21
WS4	14.13	NA	72.78	NA	0.25	NA
WS4A	NA	0.47	NA	82.41	NA	0.03
WS4B	NA	2.52	NA	82.54	NA	0.09
WS4C	NA	1.80	NA	80.00	NA	0.13
WS4D	NA	3.38	NA	74.00	NA	0.19

NA

73.17

NA

NA

NA

NA

76.77

73.31

NA

78.08

70.35

80.00

75.50

77.10

NA

0.22

NA

NA

NA

NA

0.28

TABLE 1 - COMPARISON OF WATERSHEDS, CURVE NUMBERS AND TRAVEL TIMES:

DESCRIPTION OF THE DESIGN POINTS:

NA

5.89

NA

NA

NA

NA

15.63

WS4E

WS5

WS5A

WS5B

WS5C

WS5D

WS6

The design points that are evaluated in this report are described as follows:

4.49

NA

1.01

3.44

3.16

0.38

15.71

Design Point 1 - This design point is in the NYCDEP watershed area and is located in the southeast corner of the site. This design point represents the runoff from the site just before it enters the existing stream at this location. This design point is analyzed as it represents the net peak flows from existing Watershed 1. In the proposed case this design point represents the net peak flows from Watersheds 1A, 1B, 1C, and 1D. In the proposed case, 7.5 inches of runoff off the rooftops in Watersheds 1A and 1C, is captured in drywells.

Design Point 2 - This design point is located at the southwest corner of the site at the intersection of two existing field stone walls. This design point is analyzed as it represents the net peak flows from existing and proposed Watershed 2. In the proposed case, 7.5 inches of runoff off the rooftops is captured in drywells.

Design Point 3 - This design point is located on the west side of the site at the intersection of two existing field stone walls. This design point is analyzed as it represents the net peak flows from Watershed 3 in the existing and the proposed case. In the proposed case, 1.3 inches of runoff off the rooftops is captured in drywells.

Design Point 4 - This design point is located in the northwest corner of the site, within an existing drainage way. This design point is analyzed as it represents the net peak flows from existing Watershed 4. In the proposed case this design point represents the net peak flows from Watersheds 4A, 4B, 4C, 4D, and 4F.

Design Point 5 - This design point is located in the northeast corner of the site at an existing depression were the runoff leaves the site. This design point is analyzed as it represents the net peak flows from Watershed 5 in the existing case. In the proposed case, this design point represents the net peak flows from Watersheds 5A, 5B, 5C, and 5D. In both, the existing and proposed cases, the eastern property line is modeled as a linear design point. In the proposed case, 7.5 inches of runoff off the rooftops in Watershed 5B, is captured in drywells.

Design Point 6 This design point is located just upstream of the culvert which conveys runoff from the northeast portion of the site. Existing watersheds 3, 4, and 6 are summed at this location. Watershed 6 consists predominately of offsite areas. In the proposed case, this design point represents the runoff from watersheds 3, 4B, 4C, 4D, and 6.

DISCUSSION:

The proposed stormwater management plan permits the control of stormwater for all storms in the 2 year through 100-year frequency of occurrence. For example, at Design Point DP1, the peak flow after development is reduced from 33 cfs to 32 cfs during the 100-year storm. At Design Point DP3, the peak flow after development is maintained at 15 cfs during the 100-year storm.

Since the flows after development match the flows prior to development for the 10 and 100 year storms, the NYSDEC criteria for Overbank Flood (Q_p) and Extreme Storm (Q_f) are met. Channel Protection (Cp_v) criteria is also met by detaining the proposed condition 1 year storm for a minimum of 24 hours.

TABLE 2- COMPARISON OF EXISTING AND PROPOSED PEAK FLOWS AT OFFSITE DESIGN POINTS

DESIGNA						1
<u>DESIGN</u> <u>POINT</u>	STORM EVENT (YR)	EXISTING PEAK FLOW (CFS)	PROPOSED PEAK FLOW (CFS)	NET CHANGE (CFS)	PERCENT CHANGE OVER PRIOR CONDITIONS	CHECK
DP1	100	33	32	-1	-3.0	OK
	50	29	27	-2	-6.9	OK
	25	22	21	-1	-4.5	OK
	10	15	14	-1	-6.7	OK
	5	12	10	-2	-16.7	OK
	2	6	3	-3	-50.0	OK
DP2	100	4.8	4.6	-0.2	-4.2	OK
	50	4.3	4.1	-0.2	-4.7	OK
	25	3.2	3.1	-0.1	-3.1	OK
	10	2.2	2.1	-0.1	-4.5	OK
	5	1.8	1.7	-0.1	-4.5	OK
	2	0.9	0.9	0.0	0.0	OK
DP3	100	15	15	0	0.0	OK
	50	14	13	-1	-7.1	OK
	25	11	10	-1	-9.1	OK
	10	8	8	0	0.0	OK
	5	7	6	-1	-14.3	OK
	2	4	4	0	0.0	OK
DP5	100	20	19	-1	-5.0	OK
	50	18	16	-2	-11.1	OK
	25	14	10	-4	28.6	OK
	10	10	7	-3	-30.0	OK
	5	8	6	-2	-25.0	OK
	2	5	3	-2	-40.0	OK
DP6	100	112	111	-1	-0.9	OK
	50	101	100	-1	-1.0	OK
	25	80	76	-4	-5.0	OK
	10	59	52	-7	-11.9	OK
	5	50	43	-7	-14.0	OK
	2	31	26	-5	-16.1	OK

PROPOSED FLOW PATHS:

At Design Point 1 the runoff from the 2-year storm from Watershed 1B is diverted to a water quality basin that maintains or reduces the peak flows at Design Point 1. In Watersheds 1A and 1C the runoff from the 100-year storm from the roof tops of the houses is proposed to be captured in drywells. This assists maintaining peak flows at Design Point 1 as well as improving water quality.

At Design Point 2 mitigation of post-development peak flows to or below existing levels is performed by capturing 7.5 inches of runoff from the rooftops of the houses within Watershed number 2. This capture also maintains water quality to this design point.

At Design Point 3, mitigation of post-development peak flows to or below existing levels is performed by capturing 1.3 inches of runoff from the rooftops of the houses within Watershed number 3.

At Design Point 4 the mitigation of the proposed flows is accomplished by two water quality basins. The first basin is Water Quality Basin 4A an offline basin that captures in excess of the runoff from the 1.3 inch rainfall event as per NYS DEC. This basin will operate as an extended detention basin. A second basin (WQB 4B) is a flow-through basin that treats the runoff from Watersheds 4B, 4C, and 4D. This basin captures in excess of the runoff from the 1.3" rainfall event and releases it slowly as per NYS DEC guidelines. In order to simplify data analysis, all runoff from watershed 4 is assumed to flow to the Route 6 crossing at DP6.

At Design Point 5 the runoff from Watersheds 5A and 5C are treated in Water Quality Basin 5A, a flow-through water quality basin. Water Quality Basin 5A captures in excess of the runoff from the 1.3" rainfall event and releases it slowly as per NYSDEC guidelines. Mitigation of post-development peak flows is also performed by capturing 1.3 inches of runoff from the rooftops of the houses within Watershed 5B.

At Design Point 6 the runoff from watersheds 4A, 4B, 4C, 4D, 4E, and 6 are summed.

WATER QUALITY:

First flush capture and treatment through successive structural and natural devices will provide long term treatment of runoff in keeping with the intent NYS DEC and NYC DEP rules. For detailed pollutant loading estimates, see the separate report entitled "Pollutant Loading Analysis".

CONCLUSION:

As the proposed stormwater management plan allows for the maintenance of peak flows to existing conditions or reductions in peak flows for a wide variety of storms at all design points studied, there should be no adverse impacts due to stormwater, either on-site or off-site, as a result of the proposed site development.

Submitted By:

EXISTING CUP	RVE NUMB	ERS				
WATERSHED	1					
	AREA			SOIL	CURVE	
SUBAREA	AC.	LAND USE	CONDITION	GROUP	NUMBERS	PRODUCT
1	3.58	WOODS	GOOD	В	55	196.79
2	1.45	WOODS	GOOD	D	77	111.50
3	0.09	WOODS	GOOD	С	70	6.51
4	2.48	MEADOW	GOOD	С	71	175.94
5	4.3	WOODS	GOOD	С	70	300.72
6	0.17	MEADOW	GOOD	В	58	9.98
7	1.33	WOODS	GOOD	В	55	73.37
TOTAL	13.40				65.29	874.80
WATERSHED	<u> </u> 2					
	AREA			SOIL	CURVE	
SUBAREA	AC.	LAND USE	CONDITION	GROUP	NUMBERS	PRODUCT
1	1.61	WOODS	POOR	В	66	106.33
TOTAL	1.61				66.00	106.33
WATERSHED	1 3					
	AREA			SOIL	CURVE	
SUBAREA	AC.	LAND USE	CONDITION	GROUP	<u>NUMBERS</u>	PRODUCT
1	1.44	PASTURE	FAIR	С	79	113.44
2	0.83	WOODS	POOR	С	77	63.53
3	0.86	PASTURE	FAIR	С	79	68.26
4	0.95	PASTURE	FAIR	В	69	65.55
5	0.17	WOODS	POOR	В	66	11.42
TOTAL	4.25				75.85	322.19
WATERSHED -	1					
	AREA			SOIL	CURVE	
<u>SUBAREA</u>	AC.	LAND USE	CONDITION	<u>GROUP</u>	<u>NUMBERS</u>	PRODUCT
1	0.16	IMPERVIOUS		С	98	15.88
2	0.2	PASTURE	GOOD	С	74	14.50
3	0.47	WOODS	GOOD	С	70	32.69
4	7.7	PASTURE	GOOD	С	74	570.02
5	4.83	WOODS	GOOD	С	70	338.03
6	0.35	PASTURE	GOOD	С	74	25.53
7	0.43	PASTURE	GOOD	С	74	31.52
TOTAL	14.13				72.78	1028.18

WATERSHED 5						
	AREA			SOIL	CURVE	
<u>SUBAREA</u>	AC.	LAND USE	CONDITION	<u>GROUP</u>	<u>NUMBERS</u>	<u>PRODUCT</u>
1	0.09	WOODS	FAIR	D	79	7.43
2	0.01	WOODS	FAIR	С	73	0.66
3	0.02	PASTURE	GOOD	D	80	1.28
4	0	PASTURE	GOOD	D	80	0.16
5	2.5	PASTURE	GOOD	С	74	184.63
6	3.1	WOODS	FAIR	С	73	226.59
7	0.17	WOODS	FAIR	В	60	10.08
TOTAL	5.89				73.17	430.83

WATERSHED 6	:					
WATERSHED				2011	0115175	
	AREA			SOIL	CURVE	
<u>SUBAREA</u>	<u>AC.</u>	LAND USE	CONDITION	GROUP	<u>NUMBERS</u>	<u>PRODUCT</u>
1	0.37	IMPERVIOUS		С	98	35.83
2	8.63	WOODS	FAIR	С	73	630.02
3	1.35	RESIDENTIAL	1/2 ACRE	С	80	107.89
4	0.18	IMPERVIOUS		С	98	17.85
5	4.88	RESIDENTIAL	1/2 ACRE	С	80	390.66
6	0.22	RESIDENTIAL	1/2 ACRE	С	80	17.83
TOTAL	15.63				76.77	1200.08

PROPOSED CI	PROPOSED CURVE NUMBERS					
WATERSH	IED 1A					
	AREA			SOIL	CURVE	
<u>SUBAREA</u>	AC.	LAND USE	CONDITION	GROUP	<u>NUMBERS</u>	<u>PRODUCT</u>
1	0.98	RESIDENTIAL	1/2 ACRE	В	70	68.90
2	0.51	RESIDENTIAL	1/2 ACRE	С	80	41.00
TOTAL	1.50				73.42	109.90
WATERSH	IED 1B					
	AREA			SOIL	CURVE	
<u>SUBAREA</u>	AC.	LAND USE	CONDITION	<u>GROUP</u>	<u>NUMBERS</u>	<u>PRODUCT</u>
1	1.01	RESIDENTIAL	1/2 ACRE	В	70	71.04
2	3.50	RESIDENTIAL	1/2 ACRE	С	80	279.76
TOTAL	4.51				77.75	350.80
WATERSH	ED 1C					
	AREA			SOIL	CURVE	
<u>SUBAREA</u>	AC.	LAND USE	CONDITION	GROUP	NUMBERS	<u>PRODUCT</u>
1	2.18	RESIDENTIAL	1/2 ACRE	С	80	174.01
2	0.88	RESIDENTIAL	1/2 ACRE	В	70	61.36
TOTAL	3.05	·			77.13	235.38

WATERSH	IED 1D					
WATERSI	AREA			SOIL	CURVE	
SUBAREA	AC.	LAND USE	CONDITION	GROUP	NUMBERS	PRODUCT
1	0.19	OPEN SPACE	GOOD	B	61	11.85
2	1.33	WOODS	GOOD	В	55	73.18
3	1.45	WOODS	GOOD	D	77	111.49
4	0.10	RESIDENTIAL	1/2 ACRE	В	70	7.22
5	0.12	RESIDENTIAL	1/2 ACRE	C	80	9.34
6	_		0.09 WOODS GOOD C	70	6.49	
7	0.36	WOODS	GOOD	C	70	25.03
TOTAL	3.64	110000	<u> </u>	J	67.14	244.59
TOTAL	3.04				07.14	244.55
WATERSI	HED 2					
	AREA			SOIL	CURVE	
<u>SUBAREA</u>	AC.	LAND USE	<u>CONDITION</u>	<u>GROUP</u>	<u>NUMBERS</u>	PRODUCT
1	1.65	RESIDENTIAL	1/2 ACRE	В	70	115.31
TOTAL	1.65				70.00	115.31
	.== .					
WATERSHED 3				0011	OUDVE	
01154554	AREA			SOIL	CURVE	
SUBAREA	<u>AC.</u>	LAND USE	CONDITION	GROUP	NUMBERS	PRODUCT
1	0.52	RESIDENTIAL	1/2 ACRE	С	80	41.21
2	0.15	OPEN SPACE	GOOD	С	74	11.29
3	0.05	WOODS	POOR	С	77	4.00
4	1.35	OPEN SPACE	GOOD	С	74	99.90
5	0.16	WOODS	POOR	С	77	12.30
6	0.55	RESIDENTIAL	1/2 ACRE	С	80	43.75
7	0.17	OPEN SPACE	GOOD	С	74	12.46
8	0.22	WOODS	POOR	С	77	17.09
9	0.83	RESIDENTIAL	1/2 ACRE	В	70	58.11
10	0.09	OPEN SPACE	GOOD	В	61	5.63
11	0.10	WOODS	POOR	В	66	6.68
TOTAL	4.19				74.56	312.43
WATERSHED) 4A					
	AREA			SOIL	CURVE	
SUBAREA	AC.	LAND USE	CONDITION	GROUP	NUMBERS	PRODUCT
1	0.11	OPEN SPACE	GOOD	C	74	8.21
2	0.13	OPEN SPACE	GOOD	С	74	9.73
3	0.16	IMPERVIOUS		C	98	16.09
4	0.06	OPEN SPACE	GOOD	C	74	4.56
TOTAL	0.47			-	82.41	38.59

WATERSH	HFD 4B					
WHENCE	AREA			SOIL	CURVE	
SUBAREA	AC.	LAND USE	CONDITION	GROUP	NUMBERS	PRODUCT
1	0.38	IMPERVIOUS	OONDITION	B	98	37.02
2	0.69	IMPERVIOUS		C	98	67.28
3	0.26	OPEN SPACE	GOOD	В	61	15.69
4	0.05	OPEN SPACE	GOOD	В	61	2.96
5	0.25	OPEN SPACE	GOOD	С	74	18.52
6	0.52	OPEN SPACE	GOOD	C	74	38.29
7	0.39	OPEN SPACE	GOOD	C	74	28.62
TOTAL	2.52	OI EN OI AGE	ОООВ		82.54	208.37
TOTAL	2.02				02.54	200.57
WATERSH	IED 4C					
VVATEROI	AREA			SOIL	CURVE	
SUBAREA	AC.	LAND USE	CONDITION	GROUP	NUMBERS	PRODUCT
1	1.80	RESIDENTIAL	1/2 ACRE	C	80	143.71
TOTAL	1.80	RESIDENTIAL	1/2 ACILL		80.00	143.71
TOTAL	1.00				80.00	143.71
WATERSHED -	1 4D					
WATEROILE	AREA			SOIL	CURVE	
SUBAREA	AC.	LAND USE	CONDITION	GROUP	NUMBERS	PRODUCT
<u>30BARLA</u> 1	3.38	OPEN SPACE	GOOD	C	74	250.25
TOTAL	3.38	OI LIN SI ACL	ОООВ	<u> </u>	74.00	250.25
TOTAL	3.30				74.00	250.25
WATERSH	HED 4F					
WITERO	AREA			SOIL	CURVE	
SUBAREA	AC.	LAND USE	CONDITION	GROUP	NUMBERS	PRODUCT
1	0.35	IMPERVIOUS	CONDITION	C	98	34.04
2	0.02	MEADOW	GOOD	C	71	1.51
3	0.02	WOODS	GOOD	C	70	1.50
4	0.02	OPEN SPACE	GOOD	С	74	15.37
4 5	0.25	OPEN SPACE	GOOD	C	74	18.58
6	0.23	MEADOW	GOOD	С	71	10.04
7	0.14	WOODS	GOOD	C	70	38.66
8	2.36	MEADOW	GOOD	C	71	167.82
9	1	1	GOOD	C	70	
9 10	0.36	WOODS		C	70	24.97
	0.20	OPEN SPACE	GOOD			14.63
11	0.03	WOODS	GOOD	С	70	2.31
TOTAL	4.49				73.31	329.44

WATERSHED !	5A					
WITEITONES	AREA			SOIL	CURVE	
SUBAREA	AC.	LAND USE	CONDITION	GROUP	NUMBERS	PRODUCT
1	0.32	OPEN SPACE	GOOD	С	74	23.95
2	0.69	RESIDENTIAL	1/2 ACRE	С	80	54.82
TOTAL	1.01				78.08	78.77
WATERSH	ED 5B					
	AREA			SOIL	CURVE	
SUBAREA	AC.	LAND USE	CONDITION	GROUP	NUMBERS	PRODUCT
1	3.32	RESIDENTIAL	1/2 ACRE	В	70	232.15
2	0.12	RESIDENTIAL	1/2 ACRE	С	80	9.65
TOTAL	3.44				70.35	241.81
WATERSH	ED 5C					
	AREA			SOIL	CURVE	
SUBAREA	AC.	LAND USE	CONDITION	GROUP	NUMBERS	PRODUCT
1	3.16	RESIDENTIAL	1/2 ACRE	С	80	252.43
TOTAL	3.16				80.00	252.43
WATERSH	ED 5D					
	AREA			SOIL	CURVE	
<u>SUBAREA</u>	AC.	LAND USE	CONDITION	GROUP	<u>NUMBERS</u>	<u>PRODUCT</u>
1	0.01	WOODS	FAIR	С	73	0.64
2	0.05	WOODS	FAIR	D	79	4.30
3	0.04	OPEN SPACE	GOOD	D	80	3.43
4	0.01	WOODS	FAIR	С	73	0.76
5	0.14	PASTURE	GOOD	С	74	10.49
6	0.01	PASTURE	GOOD	D	80	0.85
7	0.11	OPEN SPACE	GOOD	С	74	8.42
TOTAL	0.38				75.50	28.89
WATERSH	HED 6					
	AREA			SOIL	CURVE	
<u>SUBAREA</u>	AC.	LAND USE	CONDITION	GROUP	<u>NUMBERS</u>	<u>PRODUCT</u>
1	1.42	WOODS	FAIR	С	73	103.92
2	0.37	IMPERVIOUS		С	98	35.83
3	1.35	RESIDENTIAL	1/2 ACRE	С	80	107.89
4	0.40	IMPERVIOUS		С	98	39.29
5	4.88	RESIDENTIAL	1/2 ACRE	С	80	390.66
6	7.06	WOODS	FAIR	С	73	515.67
7	0.22	RESIDENTIAL	1/2 ACRE	С	80	17.83
TOTAL	15.71				77.10	1211.09

EXISTING TRAVEL TIMES							
SCS TR-55 TRAVEL TIME							
COMPUTATIONS							
WATERSHED 1							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
150.00	609.0	605.9	2.067	0.240	3.300		0.320
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
450.26	605.9	564.0	9.306				0.025
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
151.42	564.0	562.0	1.321				0.023
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
174.53	562.0	534.0	16.043				0.008
SHALLOW CONCENTRATED	1						
FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
387.69	534.0	531.0	0.774				0.076
TOTAL			TOTAL	AVERAGE			TOTAL
LENGTH			DELTA Y	VELOCITY			TRAVEL T
(FT)	1		(FT)	(FPS)			(HRS)
1313.90	1		78.000	0.809			0.451
	1	İ					
	1					LAG	0.27
	1						
WATERSHED 2	1						
	1	İ					
SHEET FLOW (L.T. 150 FT)		İ					
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
150.00	615.0	607.5	5.000	0.240	3.300		0.225

SHALLOW CONCENTRATED							
FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
455.76	607.5	596.0	2.523				0.049
TOTAL			TOTAL	AVERAGE			TOTAL
LENGTH			DELTA Y	VELOCITY			TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
605.76			19.000	0.614			0.274
						LAG	0.16
WATERSHED 3							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
150.00	612.5	608.5	2.667	0.240	3.300		0.289
SHALLOW CONCENTRATED							
FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
556.95	608.5	585.0	4.219				0.047
TOTAL			TOTAL	A)/EDAOE			TOTAL
TOTAL			TOTAL	AVERAGE			TOTAL
LENGTH			DELTA Y	VELOCITY			TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
706.95			27.500	0.585			0.335
						LAG	0.20
WATERSHED 4							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
150.00	612.1	607.8	2.867	0.240	3.300		0.280
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
1301.44	607.8	530.0	5.978				0.092
SHALLOW CONCENTRATED							
SI IALLOW CONCENTRATED	1	1	1	ĺ		1	

FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
307.76	530.0	526.5	1.137				0.050
TOTAL			TOTAL	AVERAGE			TOTAL
LENGTH			DELTA Y	VELOCITY			TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
1759.20			85.600	1.158			0.422
						LAG	0.25
WATERSHED 5							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
122.00	609.0	604.0	4.098	0.240	3.300		0.206
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
28.00	604.0	602.1	6.786	0.240	3.300		0.052

WATEROUER							
WATERSHED 6							
OUEET ELONG (L. T. 450 ET)							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
150.00	604.0	601.0	2.000	0.240	3.300		0.324
SHALLOW CONCENTRATED							
FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
505.00	601.0	594.0	1.386				0.074
SHALLOW CONCENTRATED							
FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
1051.00	594.0	516.0	7.422				0.066
TOTAL			TOTAL	AVERAGE			TOTAL
LENGTH			DELTA Y	VELOCITY			TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
1706.00			0.000	1.021			0.464
						LAG	0.28

SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)						
LENGTH	ELEV1	ELEV2	SLOPE			TRAVEL
(FT)			PERCENT			TIME
1375.31	602.1	536.2	4.792			0.108
TOTAL			TOTAL	AVERAGE		TOTAL
LENGTH			DELTA Y	VELOCITY		TRAVEL T
(FT)			(FT)	(FPS)		(HRS)
1525.31			72.800	1.157		0.366
					LAG	0.22

		1	1				_
PROPOSED TRAVEL TIMES							
SCS TR-55 TRAVEL TIME							
COMPUTATIONS							
WATERSHED 1A							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
150.00	602.5	584.0	12.333	0.240	3.300		0.156
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
229.72	584.0	564.0	8.706				0.013
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
126.82	564.0	563.0	0.789				0.025
TOTAL			TOTAL	AVERAGE			TOTAL
LENGTH			DELTA Y	VELOCITY			TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
506.54			39.500	0.724			0.194
500.34			39.500	0.724			0.194
						LAG	0.12

WATERSHED 1B							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
150.00	612.1	603.0	6.067	0.240	3.300		0.208
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
305.97	603.0	563.0	13.073				0.015
TOTAL			TOTAL	AVERAGE			TOTAL
LENGTH			DELTA Y	VELOCITY			TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
455.97			49.100	0.570			0.222
						LAG	0.13
WATERSHED 1C	 						
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
150.00	611.0	593.0	12.000	0.240	3.300		0.158
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
346.43	593.0	568.0	7.216				0.022
TOTAL			TOTAL	AVERAGE			TOTAL
LENGTH			DELTA Y	VELOCITY			TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
496.43			43.000	0.764			0.180
						LAG	0.11

WATERSHED 1D							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
150.00	592.5	574.0	12.333	0.240	3.300		0.156
SHALLOW CONCENTRATED							
FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
203.12	574.0	536.0	18.708				0.008
OLIALI OM CONCENTRATED							
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)	LLLVI	LLLVZ	PERCENT				TIME
227.58	536.0	531.0	2.197				0.026
221:30	550.0	551.0	2.197				0.020
TOTAL	-		TOTAL	AVERAGE			TOTAL
LENGTH	-		DELTA Y	VELOCITY			TRAVEL T
(FT) 580.70			(FT) 61.500	(FPS) 0.845			(HRS) 0.191
560.70			61.500	0.045			0.191
						LAG	0.11
						LAG	0.11
WATERSHED 2							
WATERSHED 2							
SHEET FLOW (L.T. 150 FT)							
` '		EL EV/2	CLODE	NAANINIINIO	2 VD DDD		TDAY/EL
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)	045.0	007.0	PERCENT	N	(INCHES)		TIME
150.00	615.0	607.0	5.333	0.240	3.300		0.219
CHALLOW CONCENTRATED	_						
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
445.54	607.0	596.0	2.469				0.049
	557.5	000.0	2.100				3.010
TOTAL	+	1	TOTAL	AVERAGE			TOTAL
LENGTH		†	DELTA Y	VELOCITY			TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
595.54			19.000	0.618			0.268
090.04			13.000	0.010			0.200
						LAG	0.16
1	1	1	I	1		LAG	3.10

WATERSHED 3							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
150.00	614.1	610.0	2.733	0.240	3.300		0.286
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
370.70	610.0	598.0	3.237				0.035
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)	İ		PERCENT				TIME
333.36	598.0	585.0	3.900				0.029
TOTAL			TOTAL	AVERAGE			TOTAL
LENGTH			DELTA Y	VELOCITY			TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
854.06			29.100	0.677			0.350
						LAG	0.21

WATERSHED 4A							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
24.72	572.0	564.0	32.362	0.240	3.300		0.025
SHALLOW CONCENTRATED							
FLOW (PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
186.29	564.0	549.0	8.052				0.009
OPEN CHANNEL FLOW - PIPE SECTION							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	DIAMETER		TRAVEL
(FT)	·		PERCENT	N	(INCHES)		TIME
125.08	546.5	543.0	2.798	0.040	15.000		0.012
TOTAL	040.0	040.0	TOTAL	AVERAGE	10.000		TOTAL
LENGTH			DELTA Y	VELOCITY			TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
311.37			18.500	1.871			0.046
011.07			10.000	1.071			0.040
						LAG	0.03
						2710	0.00
WATERSHED 4B							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
79.05	568.0	562.0	7.590	0.240	3.300		0.114
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
27.91	562.0	555.0	25.081	0.240	3.300		0.031
		1					
TOTAL		1	TOTAL	AVERAGE		İ	TOTAL
LENGTH			DELTA Y	VELOCITY		İ	TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
106.96			13.000	0.206			0.145
						LAG	0.09
t					·		

WATERSHED 4C							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
150.00	592.0	581.0	7.333	0.240	3.300		0.193
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
396.17	581.0	549.5	7.951				0.024
TOTAL			TOTAL	AVERAGE			TOTAL
LENGTH			DELTA Y	VELOCITY			TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
546.17			42.500	0.700			0.217
						LAG	0.13

	1	I	ı				1
WATERSHED 4D							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
134.26	610.1	606.0	3.054	0.240	3.300		0.250
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
15.74	606.0	605.0	6.353	0.240	3.300		0.034
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
499.80	605.0	575.9	5.822				0.036
TOTAL			TOTAL	AVERAGE			TOTAL
LENGTH			DELTA Y	VELOCITY			TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
649.80			34.200	0.565			0.320
						LAG	0.19

WATERSHED 4E							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
150.00	578.2	569.5	5.800	0.240	3.300		0.212
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
687.44	569.5	527.0	6.182				0.048
TOTAL			TOTAL	AVERAGE			TOTAL
LENGTH			DELTA Y	VELOCITY			TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
837.44			51.200	0.897			0.259
						LAG	0.16

WATERSHED 5A							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
150.00	584.2	571.0	8.800	0.240	3.300		0.179
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
65.91	571.0	564.0	10.621				0.003
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
40.18	564.0	551.0	32.354				0.001
TOTAL			TOTAL	AVERAGE			TOTAL
LENGTH			DELTA Y	VELOCITY			TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
256.09			33.200	0.387			0.184
						LAG	0.11

		1	1			1	1
WATERSHED 5B							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
150.00	602.0	582.0	13.333	0.240	3.300		0.152
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
963.18	582.0	555.5	2.751				0.100
TOTAL			TOTAL	AVERAGE			TOTAL
LENGTH			DELTA Y	VELOCITY			TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
1113.18			46.500	1.229			0.252
						LAG	0.15

WATERSHED 5C							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
136.94	608.0	602.5	4.016	0.240	3.300		0.228
SHALLOW CONCENTRATED FLOW (PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
147.45	602.2	601.0	0.814				0.022
OPEN CHANNEL FLOW - PIPE SECTION							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	DIAMETER		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
666.12	597.0	563.0	5.104	0.040	36.000		0.027
TOTAL			TOTAL	AVERAGE			TOTAL
LENGTH			DELTA Y	VELOCITY			TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
950.51			40.700	0.954			0.277
						LAG	0.17

WATERSHED 5D							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
58.16	556.0	545.0	18.913	0.240	3.300		0.062
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
112.68	545.0	536.2	7.810				0.007
TOTAL			TOTAL	AVERAGE			TOTAL
LENGTH			DELTA Y	VELOCITY			TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
170.84			8.800	0.690			0.069
						LAG	0.04

WATERSHED 6							
SHEET FLOW (L.T. 150 FT)							
LENGTH	ELEV1	ELEV2	SLOPE	MANNING	2 YR PRP		TRAVEL
(FT)			PERCENT	N	(INCHES)		TIME
150.00	604.0	601.0	2.000	0.240	3.300		0.324
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
505.00	601.0	594.0	1.386				0.074
SHALLOW CONCENTRATED FLOW (UN-PAVED PATH)							
LENGTH	ELEV1	ELEV2	SLOPE				TRAVEL
(FT)			PERCENT				TIME
1051.00	594.0	516.0	7.422				0.066
TOTAL			TOTAL	AVERAGE			TOTAL
LENGTH			DELTA Y	VELOCITY			TRAVEL T
(FT)			(FT)	(FPS)			(HRS)
1706.00			0.000	1.021			0.464
						LAG	0.28

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

HEC-1 INPUT

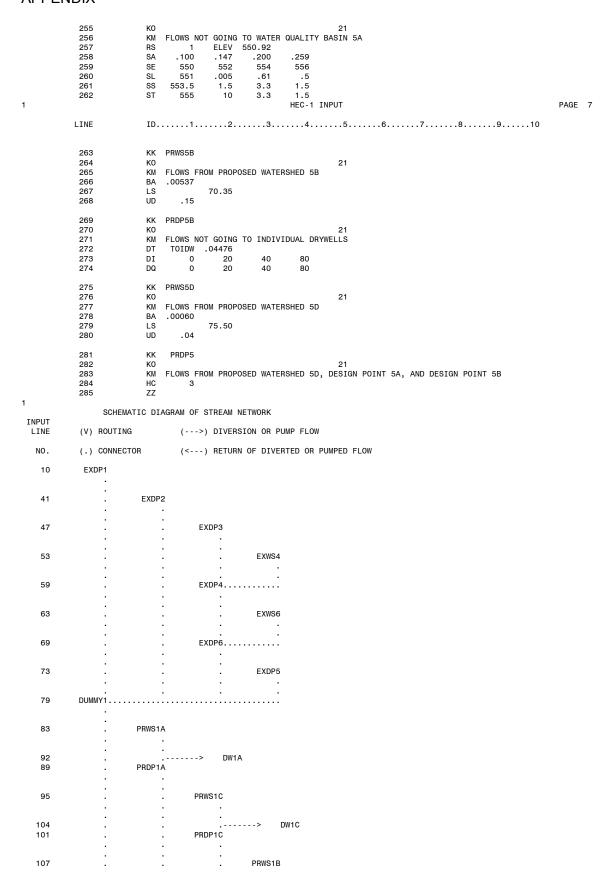
THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

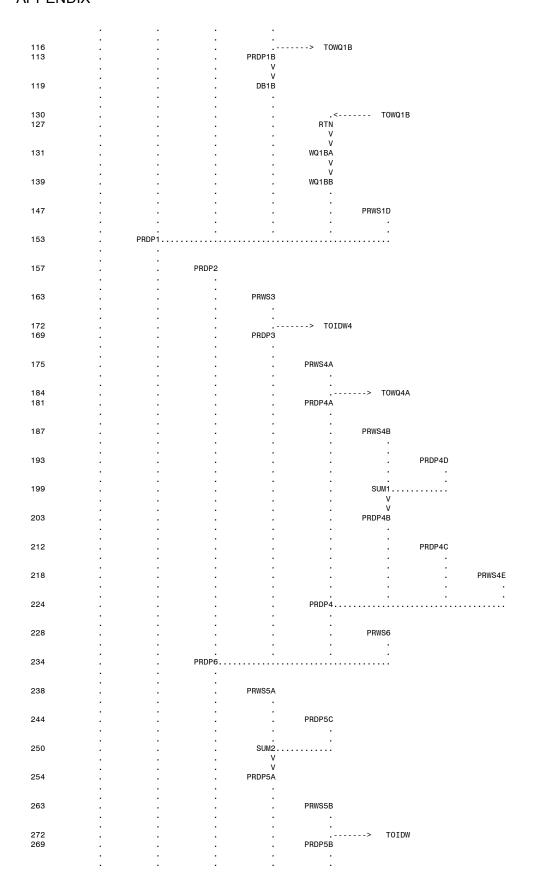
LINE $ID. \dots 1 \dots 2 \dots 3 \dots 4 \dots 5 \dots 6 \dots 7 \dots 8 \dots 9 \dots 10$ R.G. MASTROMONACO, P.E. - SANTUCCI YORKTOWN FARMS TOWN OF YORKTOWN USE SCS TYPE 3 DISTRIBUTION FOR SELECTED STORM RAINFALLS 3 FILENAME: YORKTOWN FARMS D&H 12-3-04.DAT, DATE: DECEMBER 3, 2004 TD USE SCS LAG 5 ID *DIAGRAM 6 10 5 0 000 2400 IT PREC JR .173 .373 0.466 0.60 0.666 0.80 0.933 1.00 9 IN 06 000 10 ΚK EXDP1 21 FLOWS FROM EXISTING WATERSHED 1 12 KM 13 14 PB 0.000 PC 0.001 0.002 0.003 0.004 0.005 0.006 0.007 0.008 0.009 РС 0.010 15 0.011 0.012 0.013 0.014 0.015 0.016 0.017 0.018 0.019 PC PC 16 0.020 0.021 0.022 0.023 0.024 0.026 0.027 0.028 0.029 0.030 17 0.031 0.032 0.034 0.035 0.036 0.037 0.038 0.040 0.041 0.042 18 PC PC PC PC PC PC 0.043 0.045 0.046 0.047 0.049 0.050 0.051 0.053 0.054 0.055 19 0.057 0.058 0.060 0.061 0.063 0.064 0.066 0.067 0.069 0.070 20 0.072 0.074 0.075 0.077 0.079 0.080 0.082 0.084 0.085 0.087 21 0.089 0.091 0.093 0.095 0.100 0.103 0.106 0.112 0.097 0.109 0.115 0.118 0.121 0.124 0.127 0.130 0.134 0.137 0.140 0.144 23 24 0.148 0.151 0.155 0.159 0.163 0.167 0.171 0.176 0.180 0.185 0.189 0.194 0.199 0.205 0.210 0.216 0.222 0.228 0.235 0.242 PC 25 0.276 0.298 0.250 0.258 0.266 0.287 0.312 0.328 0.363 0.416 26 PC PC PC 0.500 0.702 0.714 0.725 0.743 0.584 0.638 0.673 0.689 0.734 27 28 0.751 0.758 0.766 0.772 0.779 0.785 0.790 0.796 0.801 0.806 0.811 0.816 0.821 0.825 0.829 0.834 0.838 0.842 0.849 0.845 29 0.853 0.870 0.877 0.857 0.860 0.864 0.867 0.874 0.880 0.883 PC PC PC 30 0.886 0.889 0.892 0.895 0.898 0.900 0.903 0.906 0.908 0.910 31 32 0.911 0.929 0.913 0.930 0.917 0.933 0.920 0.936 0.924 0.915 0.919 0.922 0.925 0.927 0.932 0.942 0.935 0.938 0.941 PC PC PC 0.944 33 0.945 0.946 0.948 0.949 0.951 0.952 0.953 0.955 0.956 34 0.957 0.958 0.960 0.961 0.962 0.963 0.965 0.966 0.967 0.968 35 0.969 0.971 0.972 0.973 0.974 0.975 0.976 0.977 0.978 0.979 PC PC 36 0.981 0.982 0.983 0.984 0.985 0.987 0.988 0.986 0.989 0.990 0.991 0.992 0.993 0.994 0.995 0.996 BA LS 38 .02094 65.29 39 40 UD .27 EXDP2 41 KK 42 K0 21 FLOWS FROM EXISTING WATERSHED 2 .00252 44 ВА 45 LS 46 UD .16 HEC-1 INPUT PAGE 2

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LINE
                ID. \dots 1. \dots 2. \dots 3. \dots 4. \dots 5. \dots 6. \dots 7. \dots 8. \dots 9. \dots 10
                     EXDP3
                K0
KM
  48
                                                            21
                    FLOWS FROM EXISTING WATERSHED 3
  49
  50
                ВА
                    .00664
                LS
                              75.85
  52
                UD
                        .20
  53
                ΚK
                     EXWS4
                K0
KM
  54
                                                            21
                    FLOWS FROM EXISTING WATERSHED 4
  55
  56
                ВА
                    .02208
                LS
  58
                UD
                        .25
  59
                KK
                     EXDP4
  60
                    FLOWS FROM EXISTING WATERSHEDS 3 & 4
  61
                KM
  62
                HC
  63
                \mathsf{KK}
                     EXWS6
                K0
KM
  64
65
                                                            21
                    FLOWS FROM EXISTING WATERSHED 6
  66
                ВА
                    .02443
  67
                LS
                              76.77
                        .28
  68
                UD
                ΚK
  69
                     EXDP6
  70
71
                KO
KM
                    FLOWS FROM EXISTING WATERSHED 6 AND DP4 \,
  72
                нС
  73
                KK
K0
                     EXDP5
                                                            21
  74
75
                    FLOWS FROM EXISTING WATERSHED 5
  76
                ВА
                    .00920
  77
78
                LS
UD
                              73.17
  79
                KK
                    DUMMY1
                KM
                    SUM HYDROGRAPHS TO AVOID SCHEMATIC ERROR
  80
  81
                     4 * * * * * * *
                                        PROPOSED CONDITIONS
  83
                KK
                    PRWS1A
                    FLOWS FROM PROPOSED WATERSHED 1A
  85
                \mathsf{KM}
  86
                ВА
                    .00234
  87
                LS
                              73.42
                UD
                        .12
                                                HEC-1 INPUT
                                                                                                              PAGE 3
LINE
                ID.....1....2....3....4....5....6....7....8....9....10
                    PRDP1A
  89
                KK
                    FLOWS NOT GOING TO INDIVIDUAL DRYWELLS
  90
                K0
                \mathsf{K}\mathsf{M}
  92
                DT
                              .0860
                      DW1A
  93
                DI
                DQ
                          0
                                 20
                                          40
                                                   80
  95
                KK
                    PRWS1C
  96
                K0
                    FLOWS FROM PROPOSED WATERSHED 1C
  97
                \mathsf{K}\mathsf{M}
  98
                BA
                    .00477
                              77.13
                LS
  99
 100
                UD
                KK
 101
                    PRDP1C
                K0
 102
 103
                    FLOWS NOT GOING TO INDIVIDUAL DRYWELLS
 104
                DT
                      DW1C
                              .1721
                                                   80
 105
                DΙ
                                          40
                         0
                                 20
 106
                DQ
                KK
KO
 107
                    PRWS1B
 108
                                                            21
 109
                KM
                    FLOWS FROM PROPOSED WATERSHED 1B
 110
                ВА
                    .00706
                              77.74
                LS
 111
                UD
                        .13
 112
                ΚK
                    PRDP1B
 113
                                                            21
 114
                K0
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FLOWS NOT GOING TO WATER QUALITY BASIN 1B
 115
 116
                    TOWQ1B
                              0.66
 117
               DT
                        n
                               100
               DQ
                               100
118
                         0
               KK
 119
                     DB1B
                   FLOW ROUTED THROUGH DETENTION BASIN 1B
 120
               K0
 121
               KM
 122
               RS
                             ELEV
                                       560
               SA
SE
                                               . 0365
 123
                        Λ
                             .0365
                                      .0365
                                                       0365
                                                                .0365
 124
                      560
                            560.1
                                       562
                                                563
                                                        564
                                                                565
 125
               SL
                   560.75 .921752
                                        .61
                                                 .5
 126
               SS
                    563.5
                                     3.367
127
               ΚK
                      RTN
 128
 129
               KM
                   RETRIEVE BYPASSED FLOW
130
                                               HEC-1 INPUT
                                                                                                          PAGE 4
               ID.....1....2.....3.....4.....5.....6.....7.....8.....9....10
LINE
131
                    WQ1BA
132
133
               KO
KM
                   FLOW ROUTED THROUGH EXTENDED DETENTION BASIN 1BA
               RS
                             ELEV
                                     561.5
 134
 135
               SA
SE
                    .10879
                            .14966
                                    .19573
                                             . 24698
                      560
                               562
                                       564
 136
                                                566
                   561.58 .005454
 137
               SL
                                        .61
                                                 .5
               SS
                    564.0
                                     3.367
 139
               KK
                    WQ1BB
               K0
 140
 141
                   FLOW ROUTED THROUGH EXTENDED DETENTION BASIN 1BB
               RS
SA
 142
                              ELEV
                                       561
                    .04287
                           .06899
                                    .10030
                                             .13681
 143
               SE
                                                566
                   561.04 0.00545
 145
               SL
                                        .61
                                                 .5
                                     3.367
 146
               SS
                    564.0
                                                1.5
 147
               ΚK
                   PRWS1D
148
149
               K0
KM
                                                         21
                   FLOWS FROM PROPOSED WATERSHED 1D
 150
               ВА
                    .00569
               LS
                             67.14
152
               UD
                       .11
 153
                    PRDP1
 154
               K0
                   21 FLOWS FROM PROPOSED WATERSHED 1D, DESIGN POINT 1B, DESIGN POINT 1C, AND DESIGN
               KM
 155
               HC
156
 157
               ΚK
                    PRDP2
               K0
 158
                   21
FLOWS FROM PROPOSED WATERSHED 2 (7.5" OVER ROOFS TAKEN OUT)
               KM
 159
 160
               ВА
                             66.72
               LS
 161
               UD
                       .16
 162
               KK
 163
                    PRWS3
               K0
 164
                                                         21
 165
               KM
                   FLOWS FROM PROPOSED WATERSHED 3
 166
               ВА
                    .00655
 167
               LS
                             74.56
168
               UD
                       .21
 169
                    PRDP3
170
               K0
               KM
                   FLOWS NOT GOING TO INDIVIDUAL DRYWELLS
 171
 172
               DT
                           .04477
 173
               DI
                        0
                                20
               DQ
                         0
                                20
174
                                         40
                                                 80
                                               HEC-1 INPUT
                                                                                                          PAGE 5
               ID.\dots..1\dots..2\dots..3\dots..4\dots..5\dots..6\dots..7\dots..8\dots..9\dots..10
LINE
175
               KK
                   PRWS4A
176
177
               K0
               KM
                   FLOWS FROM PROPOSED WATERSHED 4A
               ВА
                   .00073
 179
               LS
                             82.41
               UD
                       .03
 180
 181
               ΚK
                   PRDP4A
 182
                   FLOWS NOT GOING TO WATER QUALITY BASIN 4A
 183
```

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184
                    TOWQ4A
                              .0459
 185
               DI
                                 20
20
                                          40
                                                   80
               DQ
 186
                         Ω
                                          40
                                                   80
                    PRWS4B
               KO
KM
BA
 188
                                                           21
                    FLOWS FROM PROPOSED WATERSHED 4B
 189
                    .00394
 190
 191
                LS
                              82.54
192
               UD
                        .09
 193
                ΚK
                    PRDP4D
               K0
KM
 194
                                                           21
                    FLOWS FROM PROPOSED WATERSHED 4D
 195
               BA
                    .00528
 196
 197
                LS
 198
               UD
                       .19
 199
                ΚK
                      SUM1
200
                    FLOWS FROM PROPOSED WATERSHED 4B AND DESIGN POINT4D
201
                KM
202
               HC
                KK
203
                    PRDP4B
               K0
KM
                    21
FLOWS NOT GOING TO WATER QUALITY BASIN 4B
204
205
206
                RS
                               ELEV 550.92
207
               SA
SE
                    .05968
                           .091756
552
                                      .12833
                                               17009
                                                       . 21705
208
                       550
                                         554
                                                 556
                SL
209
                       551
                               .005
                                         .61
                                                  .5
                SS
                       555
                                         3.3
                                                 1.5
                ST
                                 10
211
                       557
                                         3.3
212
                ΚK
                    PRDP4C
               KO
KM
BA
213
                    FLOWS FROM PROPOSED WATERSHED 4C
214
215
                    .00281
217
               UD
                       .13
                                                HEC-1 INPUT
                                                                                                             PAGE 6
                ID......1.....2.....3......4......5......6......7.....8......9.....10
LINE
               KK
KO
KM
                    PRWS4E
218
219
                    FLOWS FROM PROPOSED WATERSHED 4E
220
221
               BA
                    .00702
                LS
                              73.31
223
               UD
                        .16
224
                KK
                     PRDP4
225
                    FLOWS FROM PROPOSED WATERSHED 4E, DESIGN POINT 4B, AND DESIGN POINT 4C
226
                \mathsf{KM}
227
               нС
               KK
KO
KM
228
                     PRWS6
                                                           21
229
                    FLOWS FROM PROPOSED WATERSHED 6
230
231
               ВА
                    .02455
                              77.10
232
                LS
               UD
                       .28
233
234
                KK
                     PRDP6
235
236
               KO
KM
                    FLOWS FROM PROPOSED WATERSHED 6 AND DESIGN POINT 4
237
               нС
                    PRWS5A
                KK
238
               KO
KM
239
                                                           21
                    FLOWS FROM PROPOSED WATERSHED 5A
               BA
LS
241
                    .00158
                              78.08
242
243
               UD
                       .11
               KK
K0
                    PRDP5C
244
245
                                                           21
246
                KM
                    FLOWS FROM PROPOSED WATERSHED 5C
247
               ВА
                    .00493
248
               LS
UD
249
                        .17
                KK
250
                      SUM2
251
                K0
                                                           21
                    FLOWS FROM PROPOSED WATERSHED 5A AND DESIGN POINT 5C
                KM
252
254
                KK PRDP5A
```





275 PRWS5D 281 PRDP5..... (***) RUNOFF ALSO COMPUTED AT THIS LOCATION FLOOD HYDROGRAPH PACKAGE (HEC-1) U.S. ARMY CORPS OF ENGINEERS MAY 1991 VERSION 4.0.1E HYDROLOGIC ENGINEERING CENTER 609 SECOND STREET Lahey F77L-EM/32 version 5.01 DAVIS, CALIFORNIA 95616 Ralph G. Mastromonaco P.E.
RUN DATE 12/06/04 TIME 19:45:40 (916) 551-1748

> R.G. MASTROMONACO, P.E. - SANTUCCI YORKTOWN FARMS TOWN OF YORKTOWN USE SCS TYPE 3 DISTRIBUTION FOR SELECTED STORM RAINFALLS

FILENAME: YORKTOWN FARMS D&H 12-3-04.DAT, DATE: DECEMBER 3, 2004 USE SCS LAG

OUTPUT CONTROL VARIABLES 6 IO

TPRNT 5 PRINT CONTROL 0 PLOT CONTROL IPL0T O. HYDROGRAPH PLOT SCALE QSCAL

*** ERROR *** SPECIFIED START AND END DATES RESULT IN TOO MANY TIME PERIODS

HYDROGRAPH TIME DATA

NMIN 6 MINUTES IN COMPUTATION INTERVAL IDATE Ω

STARTING DATE STARTING TIME ITIME 0000

2000 NUMBER OF HYDROGRAPH ORDINATES

NDDATE O ENDING DATE ENDING TIME 0754 NDTIME ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 0.10 HOURS TOTAL TIME BASE 199.90 HOURS

ENGLISH UNITS

DRAINAGE AREA SQUARE MILES PRECIPITATION DEPTH INCHES LENGTH, ELEVATION

CUBIC FEET PER SECOND ACRE-FEET FLOW STORAGE VOLUME

SURFACE AREA ACRES

TEMPERATURE DEGREES FAHRENHEIT

JP MULTI-PLAN OPTION

1 NUMBER OF PLANS NPLAN

MULTI-RATIO OPTION JR

RATIOS OF PRECIPITATION

0.37 0.60 0.67 0.80 0.93 1.00

*** ***

Yorktown Farms Subdivision Drainage and Hydrology Report APPENDIX

1

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN				IED TO PRE RATIO 3			RATIO 6	RATIO 7	RATIO 8
					0.17	0.37	0.47	0.60	0.67	0.80	0.93	1.00
HYDROGRAPH AT +	EXDP1	0.02	1	FLOW TIME	0. 16.50	3. 12.40	6. 12.40	12. 12.40	15. 12.40	22. 12.30	29. 12.30	33. 12.30
HYDROGRAPH AT +	EXDP2	0.00	1	FLOW TIME	0. 15.10	0. 12.30	1. 12.30	2. 12.20	2. 12.20	3. 12.20	4. 12.20	5. 12.20
HYDROGRAPH AT	EXDP3	0.01	1	FLOW TIME	0. 12.40	3. 12.30	4. 12.30	7. 12.30	8. 12.30	11. 12.30	14. 12.30	15. 12.20
HYDROGRAPH AT +	EXWS4	0.02	1	FLOW TIME	0. 12.60	7. 12.40	11. 12.30	19. 12.30	23. 12.30	32. 12.30	40. 12.30	45. 12.30
2 COMBINED AT	EXDP4	0.03	1	FLOW	0.	9.	16.	26.	31.	43.	54.	60.
HYDROGRAPH AT	EXWS6	0.02	1	TIME FLOW	12.50	12.30	12.30	12.30	12.30	12.30 37.	12.30 47.	12.30 52.
2 COMBINED AT	EXDP6	0.05	1	TIME	12.50	12.40	12.40	12.30 50.	12.30 59.	12.30	12.30	12.30
HYDROGRAPH AT	EXBI O	0.00	•	TIME	12.50	12.40	12.30	12.30	12.30	12.30	12.30	12.30
+	EXDP5	0.01	1	FLOW TIME	0. 12.50	3. 12.30	5. 12.30	8. 12.30	10. 12.30	14. 12.30	18. 12.30	20. 12.30
4 COMBINED AT +	DUMMY1	0.09	1	FLOW TIME	1. 12.50	25. 12.40	42. 12.30	71. 12.30	87. 12.30	119. 12.30	152. 12.30	169. 12.30
HYDROGRAPH AT +	PRWS1A	0.00	1	FLOW TIME	0. 12.40	1. 12.20	2. 12.20	3. 12.20	3. 12.20	4. 12.20	5. 12.20	6. 12.20
DIVERSION TO +	DW1A	0.00	1	FLOW TIME	0. 12.40	1. 12.20	2. 12.20	3. 12.20	3. 12.10	3. 12.00	2. 11.90	2. 11.90
HYDROGRAPH AT +	PRDP1A	0.00	1	FLOW TIME	0. 0.10	0. 20.70	0. 13.50	2. 12.40	3. 12.30	4. 12.20	5. 12.20	6. 12.20
HYDROGRAPH AT +	PRWS1C	0.00	1	FLOW TIME	0. 12.30	2. 12.20	4. 12.20	6. 12.20	7. 12.20	9. 12.20	12. 12.10	13. 12.10
DIVERSION TO +	DW1C	0.00	1	FLOW TIME	0. 12.30	2. 12.20	4. 12.20	6. 12.10	5. 12.00	4. 11.90	3. 11.80	2. 11.70
HYDROGRAPH AT	PRDP1C	0.00	1	FLOW TIME	0. 0.10	0. 15.30	1. 12.60	5. 12.30	7. 12.20	9. 12.20	12. 12.10	13. 12.10
HYDROGRAPH AT	PRWS1B	0.01	1	FLOW TIME	0. 12.30	4. 12.20	6. 12.20	9. 12.20	11.	14. 12.20	17. 12.20	19. 12.20
DIVERSION TO	TOWQ1B	0.01	1	FLOW	0. 12.30	4.	6.	9.	11.	14.	16.	18.
HYDROGRAPH AT	PRDP1B	0.01	1	FLOW	0.	0.	0.	12.20	12.20	12.20	12.20	12.10
ROUTED TO +	DB1B	0.01	1	TIME FLOW TIME	0.10 0. 0.10	0.10 0. 0.10	0.10 0. 0.10	0. 16.20	13.60 1. 14.00	12.50 4. 12.60	7. 12.50	12.30 13. 12.40

4 COMBINED AT

			1	STAGE TIME	560.00 0.00	560.00 0.00	560.00 0.00	560.83 16.30	560.90 14.00	561.45 12.60	563.39 12.50	563.97 12.40
HYDROGRAPH AT +	RTN	0.00	1	FLOW TIME	0. 12.30	4. 12.20	6. 12.20	9. 12.20	11. 12.20	14. 12.20	16. 12.20	18. 12.10
ROUTED TO +	WQ1BA	0.00	1	FLOW TIME	0. 22.10	0. 21.30	0. 16.90	1. 13.30	2. 12.80	5. 12.40	5. 12.30	5. 12.20
			** 1	PEAK STAGES STAGE TIME	IN FEET 561.81 24.00	** 563.52 24.10	564.10 16.90	564.30 13.30	564.47 12.80	564.79 12.40	564.86 12.30	564.81 12.20
ROUTED TO +	WQ1BB	0.00	1	FLOW TIME	0. 34.00	0. 61.70	0. 40.40	0. 16.10	0. 14.90	0. 14.40	0. 14.70	0. 14.10
			** 1	PEAK STAGES STAGE TIME	IN FEET 561.17 39.60	** 561.99 85.90	562.79 63.00	563.95 18.00	563.98 16.60	563.99 16.00	563.99 15.80	563.99 15.80
HYDROGRAPH AT +	PRWS1D	0.01	1	FLOW TIME	0. 15.10	1. 12.20	3. 12.20	5. 12.20	6. 12.20	8. 12.20	11. 12.20	12. 12.20
5 COMBINED AT	PRDP1	0.02	1	FLOW TIME	0. 22.10	1. 12.20	3. 12.20	10. 12.30	14. 12.20	21. 12.20	27. 12.20	32. 12.20
HYDROGRAPH AT +	PRDP2	0.00	1	FLOW TIME	0. 15.70	0. 12.30	1. 12.30	2. 12.20	2. 12.20	3. 12.20	4. 12.20	5. 12.20
HYDROGRAPH AT +	PRWS3	0.01	1	FLOW TIME	0. 12.50	2. 12.30	4. 12.30	6. 12.30	8. 12.30	10. 12.30	13. 12.30	15. 12.30
DIVERSION TO +	TOIDW4	0.01	1	FLOW TIME	0. 12.50	2. 12.20	2. 12.00	1. 11.80	1. 11.50	1. 11.00	0. 10.40	0. 10.20
HYDROGRAPH AT +	PRDP3	0.01	1	FLOW TIME	0. 0.10	2. 12.30	4. 12.30	6. 12.30	8. 12.30	10. 12.30	13. 12.30	15. 12.30
HYDROGRAPH AT +	PRWS4A	0.00	1	FLOW TIME	0. 12.10	1. 12.10	1. 12.10	1. 12.10	1. 12.10	2. 12.10	2. 12.10	2. 12.10
DIVERSION TO +	TOWQ4A	0.00	1	FLOW TIME	0. 12.10	1. 12.10	1. 12.10	1. 12.10	1. 12.00	1. 12.00	1. 11.90	1. 11.80
HYDROGRAPH AT +	PRDP4A	0.00	1	FLOW TIME	0. 0.10	0. 21.10	0. 13.50	1. 12.30	1. 12.20	2. 12.10	2. 12.10	2. 12.10
HYDROGRAPH AT +	PRWS4B	0.00	1	FLOW TIME	1. 12.20	3. 12.10	4. 12.10	6. 12.10	7. 12.10	9. 12.10	11. 12.10	12. 12.10
HYDROGRAPH AT +	PRDP4D	0.01	1	FLOW TIME	0. 12.40	2. 12.30	3. 12.30	5. 12.30	6. 12.30	8. 12.20	11. 12.20	12. 12.20
2 COMBINED AT	SUM1	0.01	1	FLOW TIME	1. 12.20	5. 12.20	7. 12.20	11. 12.20	13. 12.20	17. 12.20	21. 12.20	23. 12.20
ROUTED TO +	PRDP4B	0.01	1	FLOW TIME	0. 23.90	0. 20.80	1. 14.80	3. 12.70	5. 12.50	10. 12.40	15. 12.40	17. 12.40
			** 1	PEAK STAGES STAGE	IN FEET 551.65	** 554.85	555.18	555.57	555.84	556.33	556.73	556.89
HYDROGRAPH AT				TIME	24.00	24.10	15.20	12.70	12.50	12.40	12.40	12.40
+	PRDP4C	0.00	1	FLOW TIME	0. 12.30	2. 12.20	3. 12.20	4. 12.20	4. 12.20	6. 12.20	7. 12.20	8. 12.20
HYDROGRAPH AT +	PRWS4E	0.01	1	FLOW TIME	0. 12.40	3. 12.30	4. 12.20	7. 12.20	9. 12.20	12. 12.20	15. 12.20	17. 12.20

+	PRDP4	0.02 1	FLOW TIME	0. 12.30	4. 12.20	7. 12.20	11. 12.20	14. 12.20	25. 12.30	35. 12.30	40. 12.30
HYDROGRAPH AT +	PRWS6	0.02 1	FLOW TIME	1. 12.50	10. 12.40	15. 12.40	24. 12.30	29. 12.30	38. 12.30	48. 12.30	53. 12.30
4 COMBINED AT	PRDP6	0.05 1	FLOW TIME	1. 12.50	16. 12.30	26. 12.30	43. 12.30	52. 12.30	76. 12.30	100. 12.30	111. 12.30
HYDROGRAPH AT +	PRWS5A	0.00 1	FLOW TIME	0. 12.30	1. 12.20	1. 12.20	2. 12.20	2. 12.20	3. 12.20	4. 12.10	4. 12.10
HYDROGRAPH AT	PRDP5C	0.00 1	FLOW TIME	0. 12.30	3. 12.20	4. 12.20	6. 12.20	7. 12.20	10. 12.20	12. 12.20	13. 12.20
2 COMBINED AT	SUM2	0.01 1	FLOW TIME	0. 12.30	4. 12.20	6. 12.20	8. 12.20	10. 12.20	13. 12.20	16. 12.20	17. 12.20
ROUTED TO +	PRDP5A	0.01 1	FLOW TIME	0. 24.00	0. 23.30	0. 16.90	1. 13.30	2. 12.80	5. 12.60	8. 12.50	9. 12.50
		** 1	PEAK STAGES STAGE TIME		553.14 24.10	553.62 16.90	553.86 13.30	554.05 12.80	554.48 12.60	554.85 12.50	555.02 12.50
HYDROGRAPH AT +	PRWS5B	0.01 1	FLOW TIME	0. 13.50	2. 12.30	3. 12.20	5. 12.20	6. 12.20	8. 12.20	11. 12.20	12. 12.20
DIVERSION TO +	TOIDW	0.01 1	FLOW TIME	0. 13.50	2. 12.30	2. 12.10	1. 12.00	1. 11.90	1. 11.50	1. 11.10	1. 10.90
HYDROGRAPH AT +	PRDP5B	0.01 1	FLOW TIME	0. 0.10	1. 12.50	3. 12.30	5. 12.20	6. 12.20	8. 12.20	11. 12.20	12. 12.20
HYDROGRAPH AT +	PRWS5D	0.00 1	FLOW TIME	0. 12.20	0. 12.10	1. 12.10	1. 12.10	1. 12.10	1. 12.10	2. 12.10	2. 12.10
3 COMBINED AT +	PRDP5	0.01 1 SUMMAR	FLOW TIME / OF DAM OVER	0. 12.20 RTOPPING/BF	1. 12.50 REACH AN	3. 12.30 ALYSIS FO	6. 12.20 OR STATIO	7. 12.20 N PRDP4B	10. 12.30	16. 12.30	19. 12.30
·			ARE FOR IN						N)		
PLAN 1		ELEVATION STORAGE OUTFLOW		VALUE 0.92 0. 0.		AY CREST 55.00 1. 0.		OF DAM 57.00 1. 19.			
	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXI OUTF CF	LOW 0	JRATION VER TOP HOURS	TIME OF MAX OUTFLO HOURS		.URE	
1	0.17 0.37 0.47 0.60 0.67 0.80 0.93 1.00		0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0		REACH AN				0. 0. 0. 0. 0.	00 00 00 00 00 00 00 00 00	
PLAN 1		•	INITIA	L VALUE 0.92 0. 0.	SPILLW	VAY CREST 53.50 0. 0.	TOP	OF DAM 55.00 1. 9.	.,		
	RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXI OUTF CF	LOW 0	JRATION VER TOP HOURS	TIME OF MAX OUTFLO HOURS		.URE	
	0.17 0.37	551.32 553.14	0.00	0. 0.		0. 0.	0.00 0.00	24.10 24.10		00 00	

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0.47	553.62	0.00	1.	0.	0.00	16.90	0.00
0.60	553.86	0.00	1.	1.	0.00	13.30	0.00
0.67	554.05	0.00	1.	2.	0.00	12.80	0.00
0.80	554.48	0.00	1.	5.	0.00	12.60	0.00
0.93	554.85	0.00	1.	8.	0.00	12.50	0.00
1.00	555.02	0.02	1.	9.	0.20	12.50	0.00

*** NORMAL END OF HEC-1 ***

